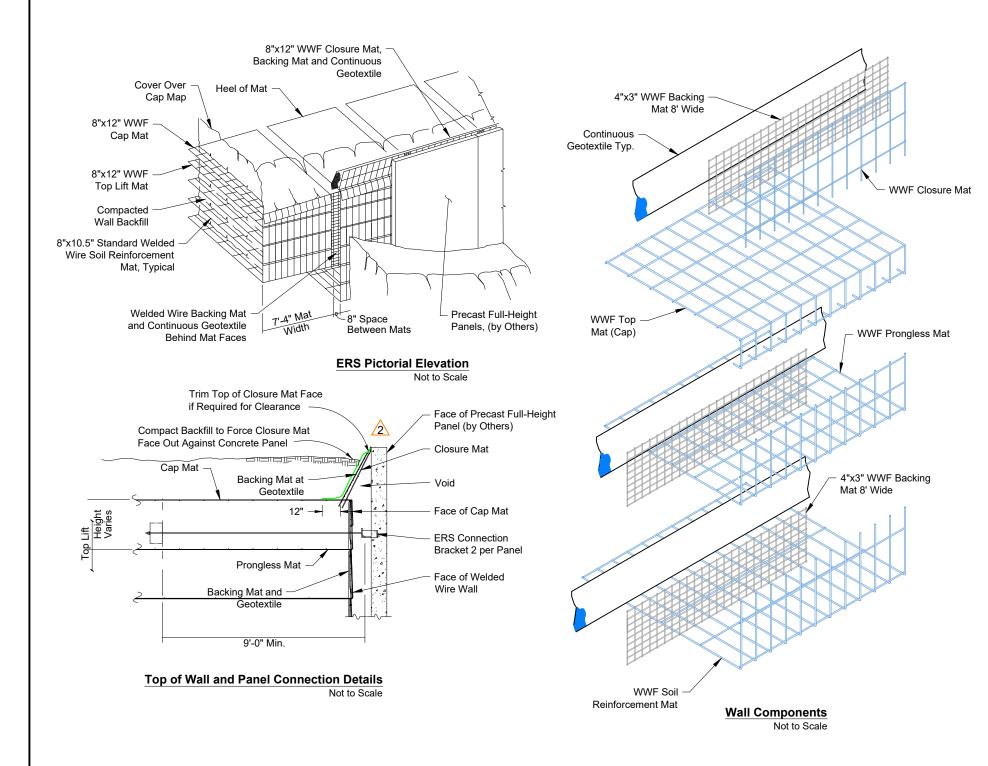


Wall Quantities 1



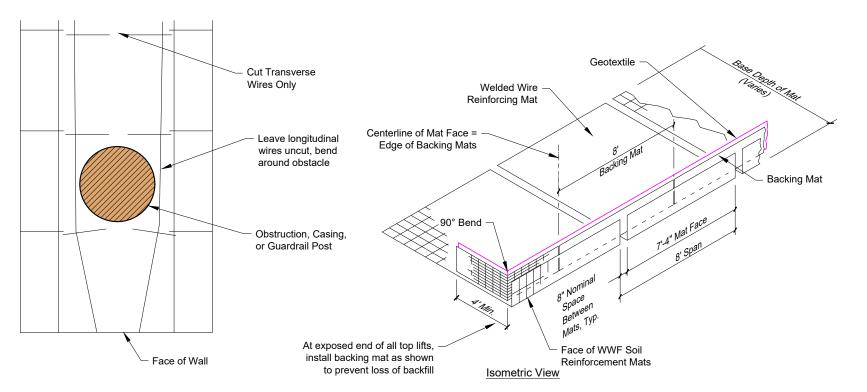


Item	Wire Spacing (Longitundianl x transverse)	Mat Type		Quantities			
			Mat Length	Access Wall	Left Wall	Right Wall	
W7.0 x 3.5	8" x 12"	Сар	14'	13			
W7.0 x 3.5	8" x 12"	Сар	13'	6			
W7.0 x 3.5	8" x 12"	Сар	11'	6	27		
W7.0 x 3.5	8" x 12"	Сар	9'	15	1	41	
W7.0 x 3.5	8" x 12"	Prongless	14'	13			
W7.0 x 3.5	8" x 12"	Prongless	13'	6			
W7.0 x 3.5	8" x 12"	Prongless	11'	6	27		
W7.0 x 3.5	8" x 12"	Prongless	9'	15	1	41	
W7.0 x 3.5	8" x 10.5	Standard	14'	94			
W7.0 x 3.5	8" x 10.5	Standard	12.25'	36			
W7.0 x 3.5	8" x 10.5	Standard	10.5'	25	104		
W7.0 x 3.5	8" x 10.5	Standard	8.75'	16		88	
Closure Mats				40	28	41	
Backing Mats (2')				260	168	177	
Hog Rings				5,100	3,200	3,100	
Face Anchors				80	56	82	
Geotextile				2,080 LF	1,344 LF	1,416 LF	
Wall Backfill				427 CY	409 CY	563 CY	
Gravel Wall Backfill				1122 CY	413 CY	105 CY	
Total Wall Backfil				1549 CY	822 CY	668 CY	

Backfill quantities are approximate and separated into all backfill (above geotextile) and gravel wall backfill (below the geotextile). Geotextile is at interface of the two wall backfill material types and is approximately at 1' to 2' above the high water elevation.

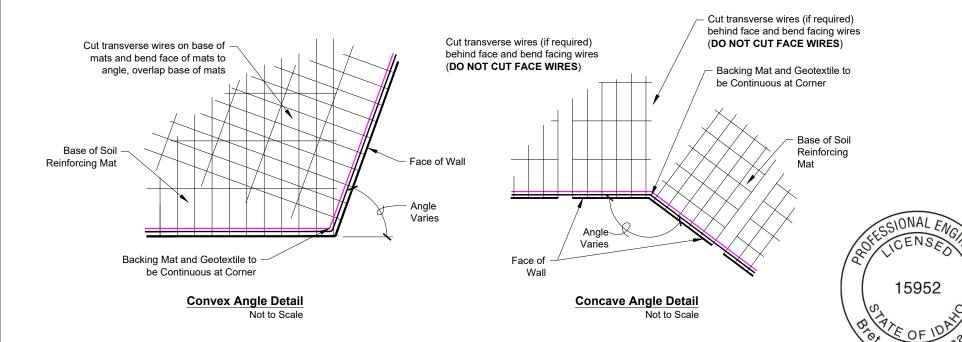


12.19.18 Submitted for Review 02.22.19 Rev. #1 per ITD Review Comments Dated 01/04/2019 03.05.19 Rev. #2 Add Geotextile and Subdrain Gravel References per ITD Comments	DESIGNED FOR: HILFIERER RETAINING WALLS 100 STREET LAW 111 107	GORDON G GEOTECHNICAL	Welded Wire MSE Walls SH-55 Bridge Over Snake River, Marsing, Idaho 83639 Federal Aid Project No: A013(387) Key No: 13387 DESIGN BY: BD DRAWN BY: TT CHECKED BY: BD APPROVED BY: BD	Welded Wire MSE Walls Details	G* PROJECT NO.: 219-016-18 HRW NO.: 180622AE SHEET NO.: 7 of 9

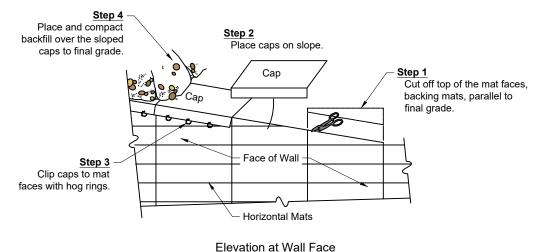


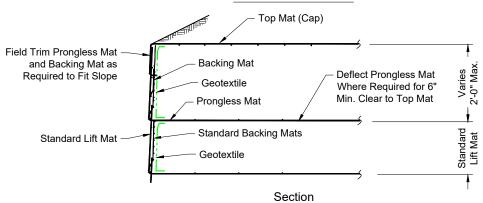
Welded Wire Wall Components With Return Backing Mat

Not to Scale



Fitting Mats to Vertical Obstructions





Installation Sequence

- 1. Place the prongless mats and backing mats for the top lifts.
- 2. Cut the prongless and backing mat faces off parallel to the slope of the final grade. Install geotextile.
- 3. Place and compact the backfill in the top lifts to parallel the final grade slope.
- 4. Place the caps on the backfill and clip them to the prongless mat faces with hog rings.
- 5. Place and compact the final cover over the cap mats.



FERENCE: Welded Wire MSE Walls Welded Wire MSE Walls PROJECT NO 12.19.18 Submitted for Review
02.22.19 Rev. #1 per ITD Review Comments Dated 01/04/2019
03.05.19 Rev. #2 Add Geotextile and Subdrain Gravel References per ITD Commen 219-016-18 HILFIKER RETAINING WALLS SH-55 Bridge Over Snake River, GORDON Details Marsing, Idaho 83639 GEOTECHNICAL 180622AE Federal Aid Project No: A013(387) ENGINEERING, INC. Key No: 13387 8 of 9 DESIGN BY: BD DRAWN BY: TT CHECKED BY: BD APPROVED BY: BD

General Notes

Introduction

- The scope of work outlined by these General Notes and Welded Wire Wall Specifications (separate document) include design and installation specifications for the Hilfiker Welded Wire MSE Walls as shown on these plans.
- 2. Grades shown herein as well as Welded Wire Wall layout and beginning and ending stations are approximate. Field conditions (both topography and geotechnical conditions exposed during construction) must be considered in determining final design configurations for construction. Conflicts between these plans and other project plans shall be resolved by Gordon Geotechnical Engineering Inc. (G²), whose decision shall be final. Geometry and layout are in general accordance with project drawings and specifications entitled "SH-55 Snake River Bridge Marsing, Federal Aid Project No. A013(387), Key No. 13387, Owyhee & Canyon Counties," by Idaho Transportation Department, dated March 2018.
- Existing and proposed facilities and utilities to be verified in field by the General Contractor.
 Conflicts that arise shall be resolved by Gordon Geotechnical Engineering Inc. of Salt Lake City, Utah, whose decision shall be final.

Welded Wire Walls

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- The work for this portion of the project shall consist of the construction of Welded Wire Walls to the lines, grades, details, and dimensions shown in these project plans and with the specifications provided for the project, as well as in conformance with all Project Plans and Specifications. Conflicts that arise shall be resolved by Gordon Geotechnical Engineering Inc. of Salt Lake City, Utah. whose decision shall be final.
- The contractor shall purchase all Welded Wire Wall materials including wire mesh reinforcing mats, and backing materials from Hilfiker Retaining Walls, 1902 Hilfiker Lane, Eureka, California, 95503, 707-443-5093. Wire mesh reinforcing mats, closure mats, and backing mats shall be constructed of hot-dipped galvanized wire (minimum 2 oz/sf).
- 3. All Welded Wire Wall installation shall be in accordance with the installation guide as manufactured by Hilfiker Retaining Walls, and these plans. Conflicts that arise shall be resolved by Gordon Geotechnical Engineering Inc. whose decision shall be final. No warranty is expressed or implied, only that the design was prepared in general accordance with the design principles and practices in use at the time this work was performed. Changes to the design or layout shall only be made with express written permission of Gordon Geotechnical Engineering Inc.
- 4. Contractor is responsible for determining exact location of welded wire walls in accordance with the intent of these plans and the overall project objectives.
- 5. Soils used as Wall Backfill 1' to 2' above the High Water Elevation shall be a well graded, non-organic, granular soil meeting all Welded Wire Wall Specifications, Project Specifications for "Reinforced Zone Backfill" (See Section S501-15A, Section G) and having the following gradation: 100% by weight passing the 4-inch sieve, 0% to 60% by weight passing the No. 40- sieve, and 0 to 15% by weight passing the No. 200 sieve. The material shall have a Plasticity Index less than 6 and meet electrochemical properties indicated in Welded Wire Wall and Project Specifications. Gradations and material properties of candidate backfill materials proposed for use may be submitted to Gordon Geotechnical Engineering for review and approval. Backfill not conforming to these specifications shall not be used without express, written permission of Gordon Geotechnical Engineering. Wall Backfill shall be compacted to 95% of the maximum dry density, as determined by ASTM D-1557 (AASHTO-T-180) compaction criteria as a minimum. This exceeds Project Specifications. Maximum lift thickness shall be 8 inches and shall be reduced, if necessary, to obtain specified density. Refer to Welded Wire Wall Specifications and Project Specifications for all applicable information.

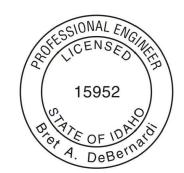
- 6. Soil used as Wall Backfill from the bottom of the wall up to a level that is 1' to 2' above the High Water Elevation shall consist of a 1.5" minus clean well graded gravel with no more than 5 percent by weight passing the 3/8" sieve. The material shall consist of durable, angular rock, and shall also comply with all electrochemical requirements and other requirements outlined in Welded Wire Wall Specifications and Project Specifications. Friable or fractured rock shall not be used. Wall Facing Gravel is to be placed in 12-inch maximum loose lifts and compacted by passing a smooth drum vibratory roller uniformly over the surface at least 4 times or as required by ITD standards for procedural compaction. Candidate materials shall have LA Abrasion test results and gradations available for approval. Alternative gravel gradations that are readily available may be submitted for review
- 7. Geotextile utilized behind the wire face, encapsulating the gravel wall backfill, behind the reinforcing mat and around subdrain shall consist of 8 oz/sy non-woven needle punched polypropylene geotextile meeting Idaho DOT Specification for Drainage Geotextile Type II.
- 8. The surface of all walls, during and after construction, shall be graded to drain. No ponding or uncontrolled flowing water shall be allowed on or around any walls, at any time.
- Contractor to provide fall protection for workers and equipment during construction in compliance
 with OSHA and any other applicable requirements. Owner shall also provide and maintain
 permanent fall protection as required by applicable building codes.
- 10. The wall foundation shall be prepared in accordance with Project Specifications and Welded Wire Wall specifications. The walls shall be established upon natural undisturbed soils. Any non-engineered fill, loose soils, etc., shall be removed and replaced with compacted granular structural fill meeting the requirements for wall backfill. Prior to placing structural replacement fill or beginning wall construction, the natural exposed subgrade shall be properly prepared in accordance with project Specifications with review by Project Geotechnical Engineer. As a minimum, the subgrade shall be proofrolled by passing a heavy steel drum roller heavy steel drum roller (minimum dynamic force of 30,000 lbs per impact and at least 1000 vibrations per minute) uniformly over the surface at least 12 times in accordance with Project Specifications. All loose or disturbed soil, organic material, trash, disturbed or generally deleterious materials or unsuitable soils shall be removed. If soft or moist conditions are encountered, the Project Geotechnical Engineer shall provide appropriate recommendations with review by G².
- 11. The walls have been designed for conventional vehicular traffic (250 psf) and soil surcharge of 270 psf (equivalent to 2'-1" maximum fill) using a 75-year service life. Walls have been designed for long term Strength 1 (static) and an Extreme Event 1 (seismic) using a pseudo-static seismic coefficient Kh = 0.12 with groundwater at the Ordinary High Water Level (Elevation 2222.90). Walls have been designed for Extreme Event II (flooding) with the High Water Level Elevation 2232.09.
- 12. The following soil properties have been utilized in the wall design:

Wall Backfill: Moist unit weight = 120 pcf, friction angle = 34 degrees.

Foundation and Retained Backfill: Moist unit weight = 125 pcf, friction angle = 34 degrees.

Retained Backfill: Moist unit weight = 125 pcf, friction angle = 33 degrees.

- 13. In accordance with Project Plans, no geomembrane has been shown over the top of the wall. A geomembrane may be required in conjunction with the use of deicing salts or magnesium chloride on the roadway as required by Idaho Department of Transportation.
- 14. G² is responsible for internal and local external stability. Bearing capacity and global stability are the responsibility of the project geotechnical engineer.



DATE DESCRIPTION

12.19.18 Submitted for Review

02.22.19 A Rev. #1 per ITD Review Comments Dated 01/04/2019

03.05.19 A Rev. #2 Add Geotextile and Subdrain Gravel References per ITD Comments

***BOX Methods Land Subdrain Gravel References per ITD Comments

***PARALAGE IN TOTAL AGENT IN TOT



Welded Wire MSE Walls SH-55 Bridge Over Snake River, Marsing, Idaho 83639 Federal Aid Project No: A013(387) Key No: 13387 Welded Wire MSE Walls
General Notes

G² PROJECT NO.:

219-016-18

HRW NO.:

180622AE

SHEET NO.:

9 of 9

DESIGN BY: BD DRAWN BY: TT CHECKED BY: BD APPROVED BY: BD